



**Shabica & Associates, Inc.**  
WE BUILD BEACHES

**RECEIVED**  
JAN 21 2016  
OFFICE OF WATER RESOURCES  
DIVISION OF RESOURCE MANAGEMENT

Federal Consistency Coordinator  
Illinois Coastal Management Program  
Illinois Department of Natural Resources  
160 N. LaSalle Street, Suite 700  
Chicago, IL 60601

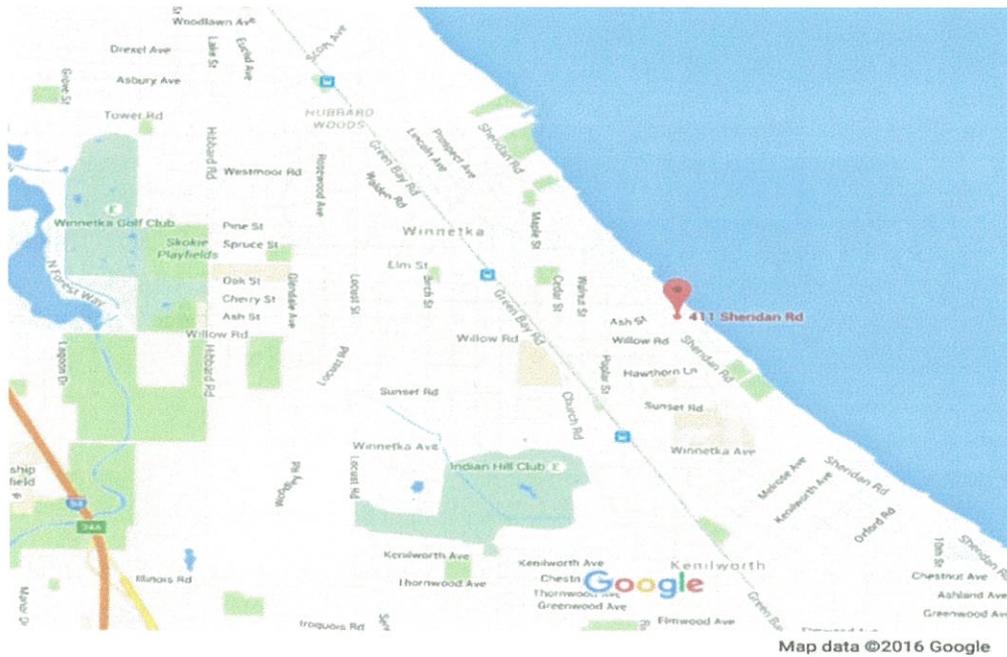
To Whom It May Concern:

January 19, 2016

In compliance with the Illinois Coastal Management Federal Consistency Review Procedures, we provide the following information for a proposed quarrystone breakwater-protected beach and sand nourishment for the properties located at 391 and 411 Sheridan Road, Winnetka, Illinois 60093, owned by Mr. Michael Hara.

**Location of Project**

The proposed quarrystone breakwater-protected beach will be built on the lakefront of the properties located at 391 and 411 Sheridan Road, Winnetka, Illinois 60093, owned by Mr. Michael Hara.



**Project Start Date and Duration**

Work will not begin until all necessary permits have been received. It is anticipated that the project can begin by August 1, 2016. This work will require approximately 14 weeks to complete.

### **Extent of Work to be Conducted**

The shoreline stabilization will be comprised of installing quarystone breakwaters that are connected to existing structures (or proposed structures already under review at 419 Sheridan) to create a cohesive bay beach system with revetments prudently located. No new work will be done further than 125' east of the bluff toe per IDNR guidelines. The project includes removal of several previously designed structures that are oddly located and in some cases impede access to Lake Michigan. Beginning at the north, a breakwater spur will be constructed extending southeast from the currently proposed breakwater at 419 Sheridan. This breakwater will have a crest of 583' and slopes of 1:1.5. Sections of quarystone revetment will be constructed lakeward of the easternmost retaining walls as necessary for shore protection. Revetment crests will be up to the elevation of the retaining walls not to exceed 588' with a slope of 1:1. A quarystone breakwater will be constructed extending north and south from the existing steel groin between the 391 and 411 lots. This breakwater will be 114' long (52' north and 62' south from the steel groin to the toe stone). The breakwater crest will be 584' with slopes of 1:1.5. Three structures on the 391 lot will be removed: the north breakwater, the concrete modular structures and the pile of armorstone in the middle of the beach that hold a perched beach. The south breakwater that extends north from the existing pier will be shortened and maintained. This structure does extend beyond 125' east from the shoreline, but its length will be reduced and the structure maintained to provide long-term stability. The crest will be reduced from a high of 588.5' to 586' tapering to 584' with slopes of 1:1.5. The revetment on the 391 lot will be maintained as necessary simply to fill void spaces and provide long-term stability. Approximately 2,962 cu. yds of clean sand will be placed as required by the IDNR.

The proposed system is designed to help retain the sandy beach, move the locus of wave energy further offshore, help reduce lakebed downcutting, and provide safe access for pedestrians to Lake Michigan.

The sand nourishment is for placement of up to 1,000 tons annually. Pending lake levels and beach conditions, sand placement will be completed as necessary to replenish sand lost to the system over time.

### **Contact Information**

All questions pertaining to this project can be submitted to:

Jon Shabica  
Shabica & Associates, Inc.  
550 Frontage Road, Suite 3735  
Northfield, IL 60093  
[jon@shabica.com](mailto:jon@shabica.com)  
847-446-1436 Tel

The proposed activity complies with Illinois' approved Coastal Management Program and will be conducted in a manner consistent with such policies.

Sincerely,

✓ Jon Shabica  
Vice President



2015 Google Earth Photo;  
see approximate  
property lines



2015 North beach cell with  
narrow beach during lake  
levels 1 ft over LWD. During  
even small lake storms, the  
concrete structures are  
vulnerable to wave attack  
and undermining.



2015 South beach cell with  
cobble beach. Concrete  
modular structures inhibit  
pedestrian access to the  
lake. Stone pile in the center  
of the beach (right side of  
photograph) hold a perched  
beach landward.



**Shabica & Associates, Inc.**  
WE BUILD BEACHES

Ms. Kathy Chernich  
East Section Chief, Regulatory Branch  
Chicago District  
U.S. Army Corps of Engineers  
231 S. LaSalle Street, Suite 1500  
Chicago, IL 60604

RECEIVED

JAN 20 2016

OFFICE OF WATER RESOURCES  
DIVISION OF RESOURCE MANAGEMENT

Dear Ms. Chernich:

January 19, 2016

Please find enclosed a permit application for shore protection and sand nourishment for the properties located at 391 and 411 Sheridan Road, Winnetka, Illinois 60093, owned by Mr. Michael Hara. The shoreline stabilization will be comprised of two bay beach cells held by quarystone breakwaters. The existing north groin (north of the north property line) is under permit review to be replaced by a quarystone breakwater for the north neighbor. The proposed project for 391 and 411 Sheridan Road will have a quarystone spur extending south from the proposed breakwater at 419 Sheridan Road. Then a quarystone breakwater will be constructed extending north and south from the existing center steel groin and the existing south breakwater will be shortened and maintained. The new breakwaters will be installed extending less than 125' offshore from the existing bluff toe to help dissipate wave energy and to narrow the gap between the north groins. The existing shore protection structures in the south cell will be removed with the exception of the south breakwater and revetment. The proposed quarystone breakwaters are designed to help reduce incident wave energy from eroding the sand and clay lakebed, to create a sustainable sand cover over the clay lakebed which reduces lakebed downcutting (deepening of the water) and helps to improve water quality caused by colloidal fines from the eroding clay being suspended in the water during storms.

Mr. Hara is also requesting a permit to allow placement of sand nourishment for 10 years after the project is complete. As the project consists of 2 beach cells, the request is for up to 1,000 tons of sand annually.

*A Design of Shoreline Erosion Protection* report has been attached to this cover letter as the coastal design specifications component of this permit. All references and figures referred to in the cover letter and the following report can be found in the Appendix.

The proposed activity complies with the approved Illinois Coastal Management Program and will be conducted in a manner consistent with such policies.

#### **Project Purpose Statement**

From conversations with Mr. and Mrs. Hara it was never their intention to do any work along the lakefront. When they purchased the lots and started working on their home in 2008, a wide sandy beach provided shore protection and allowed for an open lakefront feeling which is what they desired. The new house was built behind the state and federal jurisdictions in accordance with permitting regulations and they enjoyed the lower Lake Michigan water levels and a wide sandy beach. Within the last year, the beach has deflated to a point where the existing patio foundation is exposed to stormwaves and access to the lake and across the property is difficult to non-existent.

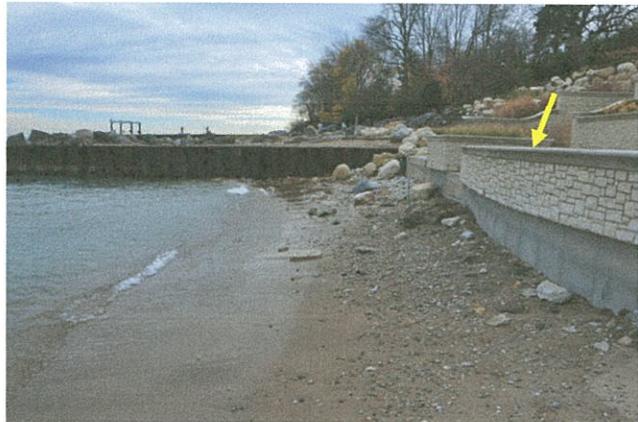
## COVER LETTER

391 & 411 Sheridan Road, Winnetka – January 19, 2016

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2009 Photo of the patio (yellow arrow)



2015 Photo (arrow indicating the same location on the patio)



2015 photos of the lakefront looking south (left) and north (right)

In September of 2015, Mr. Hara retained Shabica & Associates (SA) to help stabilize the existing beach on the Lake Michigan coastline at 391 and 411 Sheridan Road, Winnetka. This project is designed to integrate two very different beach environments while creating proper shore protection. The 391 Sheridan site currently has an array of oddly situated shore protection structures holding a perched beach and the 411 Sheridan site has a narrow beach held by a steel groin that does not provide adequate protection for the existing deck and stair system built at the west edge of the beach. With the recent deflation of the north beach and the inability of the steel groins to dissipate wave energy, it is necessary to install a shore protection system that will help to prevent additional erosion to the lakebed and beach during all lake levels. Working with the homeowner, SA designed a cohesive system that utilizes the existing groins as a foundation for a quarystone breakwater system in the north cell and cleans up the oddly designed south cell.

All beaches lose sand over time. Breakwater protected beaches typically loose sand at a slower rate than open beaches. The sand nourishment request for this project will allow the homeowner to place additional sand (up to 1,000 tons) annually or when necessary over a 10-year period. This sand nourishment then bleeds into the littoral system over time which is a benefit to downdrift properties.

### PROJECT DESCRIPTION

The shoreline stabilization will be comprised of installing quarystone breakwaters that are connected to existing structures (or proposed structures already under review at 419 Sheridan) to create a cohesive bay beach system with revetments prudently located. No new work will be done further than 125' east of the bluff toe per IDNR

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www.shabica.com

guidelines. The project includes removal of several previously designed structures that are oddly located and in some cases impede access to Lake Michigan. Beginning at the north, a breakwater spur will be constructed extending southeast from the currently proposed breakwater at 419 Sheridan. This breakwater will have a crest of 583' and slopes of 1:1.5. Sections of quarrystone revetment will be constructed lakeward of the easternmost retaining walls as necessary for shore protection. Revetment crests will be up to the elevation of the retaining walls not to exceed 588' with a slope of 1:1. A quarrystone breakwater will be constructed extending north and south from the existing steel groin between the 391 and 411 lots. This breakwater will be 114' long (52' north and 62' south from the steel groin to the toe stone). The breakwater crest will be 584' with slopes of 1:1.5. Three structures on the 391 lot will be removed: the north breakwater, the concrete modular structures and the pile of armorstone in the middle of the beach that hold a perched beach. The south breakwater that extends north from the existing pier will be shortened and maintained. This structure does extend beyond 125' east from the shoreline, but its length will be reduced and the structure maintained to provide long-term stability. The crest will be reduced from a high of 588.5' to 586' tapering to 584' with slopes of 1:1.5. The revetment on the 391 lot will be maintained as necessary simply to fill void spaces and provide long-term stability. Approximately 2,962 cu. yds of clean sand will be placed as required by the IDNR.

The proposed system is designed to help retain the sandy beach, move the locus of wave energy further offshore, help reduce lakebed downcutting, and provide safe access for pedestrians to Lake Michigan.

The sand nourishment is for placement of up to 1,000 tons annually. Pending lake levels and beach conditions, sand placement will be completed as necessary to replenish sand lost to the system over time.

### Coastal Geology

This section of coastline has historically lost sand due to lakebed downcutting especially during prolonged periods of low lake levels. Nearshore sand deposits are thin and less than one foot thick in some locations at this site (Figure 1, Appendix) and scientists estimate that the rate of lakebed erosion averages 6 inches per year (Nairn, 1997). The net result is similar to the effects of global warming and rising sea level on marine coasts. This includes deeper water nearshore, larger stormwaves and progressively narrower beaches as the nearshore lakebed continues to erode. While a beach has been present at this site during recent low lake levels, the current higher lake level has eroded the beach and the concrete deck and stairs are being impacted by stormwaves on a regular basis. If ignored, the beach will continue to deflate causing destabilization of the existing structures west of the beach.

One of the largest factors in determining the scope of a project is analyzing current lake levels and climactic conditions. Over the past few years, larger-than-normal stormwaves have impacted the shoreline of Lake Michigan, and with the higher lake level, many properties are in jeopardy of loss of structures and land. Changes in weather patterns and lake levels affect the intensity of storms. Unfortunately, it is not possible to predict future Lake Michigan lake levels and how the changing lake levels will impact the shoreline. The **Illinois State Water Survey, Prairie Research Institute** report on *Potential Impacts of Climate Change on Water Availability* ([http://www.isws.illinois.edu/iswsdocs/wsp/climate\\_impacts\\_012808.pdf](http://www.isws.illinois.edu/iswsdocs/wsp/climate_impacts_012808.pdf)) states that:

*“Scientists cannot predict future Illinois climatic conditions with confidence. The historical climate and hydrological records since the nineteenth century show that climate has changed significantly in the past and, even without human interference, could change significantly in the future.”*

The Illinois State Water Survey goes on to graph future precipitation models, illustrating conditions that are wetter or drier than previous historic extremes. Either scenario is likely to cause loss of property due to stormwave erosion from either lakebed downcutting and/or larger stormwaves. If dryer conditions are

experienced, Lake Michigan will continue to reside at or around the current low lake level and the lakebed profile will steepen due to wave-induced erosion of the glacial clay till. This steeper, nearshore lakebed profile will allow larger waves to form (see attached USACE Shore Protection Manual diagram illustrating “Dimensionless design breaker height versus relative depth at structure”). The USACE chart shows that an increased slope of the lakebed will produce larger breaking waves. On the other hand, if wetter conditions are experienced, the impacts of increased lake levels will also be seen quickly, because deeper water allows larger waves to form.

In the past two years, Lake Michigan water levels have fluctuated over 4 feet in elevation taking water levels from the all-time recorded low in January, 2013, to today’s water level around 579.0 (IGLD 1985). With that increase in water levels we are seeing the impacts of prolonged low lake levels evidenced with lakebed downcutting, deeper water nearshore, and more extreme sand fluctuation during average storm events.

### **Design Options**

The site at 391 and 411 Sheridan Road, Winnetka has been inspected and options for shore protection were determined using desktop coastal engineering, bathymetric surveys from 2012, and more than 2 decades of observations of the shoreline conditions at this site. Given the beach erosion due to higher lake levels, as well as the uncertainty of future lake levels, it is prudent to engineer and design systems that will anticipate greater lakebed downcutting, higher amounts of beach erosion, more extreme storm events with larger waves, and potential land erosion. These four options were considered:

#### **OPTION 1**

##### *Do Nothing –*

The first option of “Do Nothing” results in leaving the currently eroding beach in its existing state. The structures at the west end of the beach are vulnerable wave action and undermining. Current lakebed clay elevations at the lakeward end of the south groin may lead to destabilization of the groin by ice. This option leaves the south beach cell cluttered with structures and unsafe access to Lake Michigan.

#### **OPTION 2**

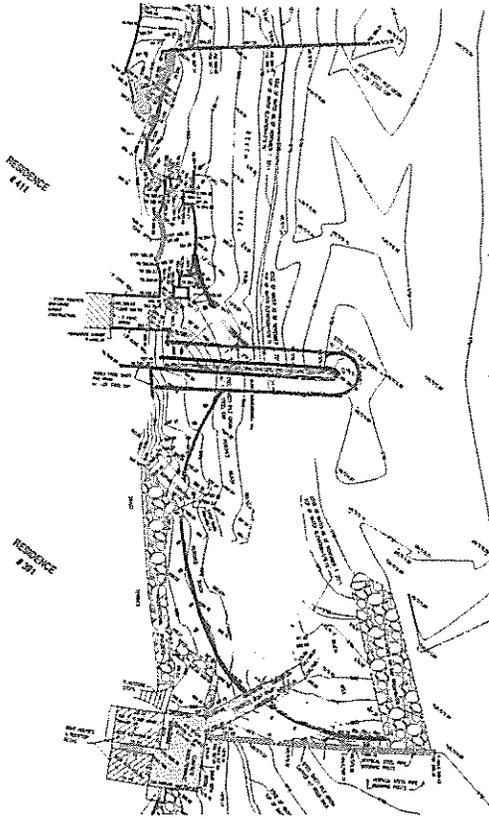
##### *Encapsulate the Middle Groin and Remove Some Structures from South Beach –*

This option shows removal of the north breakwater in the south beach cell as well as removal of the concrete modular structures and the piles of stones in the center of the south beach cell. The existing middle groin is encapsulated causing the structure to be more effective at dissipating wave energy and help to protect from toe scour at lakeward end. This option improves the ability of the groins to dissipate energy as well as help to hold sand within the system. However, as this option *does not* include breakwater spurs at the end the groins, the gap between the north and south groins remains too open to adequately protect the bluff and structures on the north beach from erosion. This option does not address lakebed erosion nor provide protection of the existing structures west of the beach.

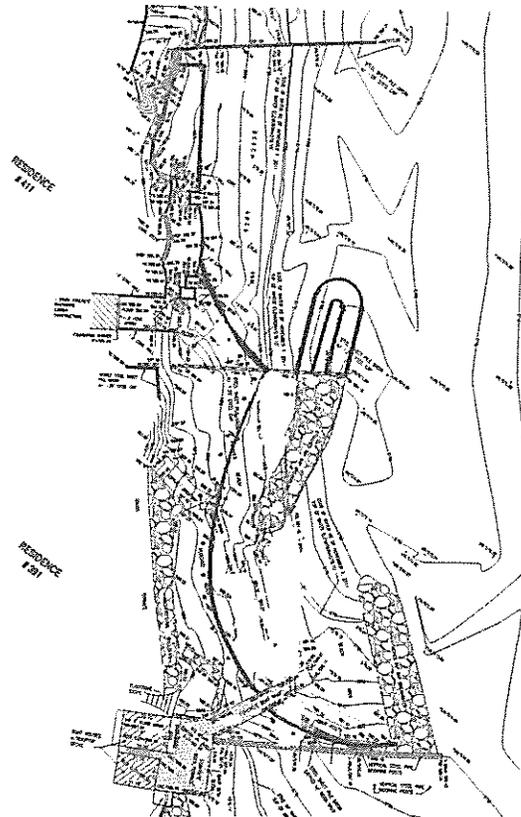
#### **OPTION 3**

##### *Construct 1 Breakwater Spur and Remove Some Structures from South Beach –*

In the north beach cell, this option shows a short breakwater spur extending north from the existing steel groin. Additionally, there is removal of the concrete modular structures in the south cell as well as removal of the pile of stone in the middle of the south beach. This option does not adequately address lakebed erosion nor protection the structures west of the beach.



**Option 2** shows groin encapsulation with stone, as well as removal of one breakwater, concrete modular structures and stone pile in middle of south beach cell



**Option 3** shows stone spur breakwater extending north from middle steel groin, as well as removal of concrete modular structures and pile of stone in middle of south beach cell

#### OPTION 4

*Proposed Option: Construct/Maintain Three Quarrystone Breakwaters to Create a Breakwater Protected Bay Beach System*

The proposed system is designed to fall within the IDNR guidelines with no new structures extending further lakeward than 125' of the bluff toe or seawall. This system will utilize the existing steel groin by connecting a quarrystone breakwater to the lakeward end of the groin, connecting a quarrystone breakwater to the proposed quarrystone breakwater currently under review for the north neighbor and shortening and maintaining the existing southernmost breakwater. This design also provides a quarrystone revetment immediately east of the existing concrete structures on the north beach and toe protection for the existing boat ramp on the south beach. This system will be filled with pre-mitigational sand, and will help protect the cohesive clay lakebed and property in a sustainable manner.

**Benefits of Sandy Beaches**

The Great Lakes represent the most important natural resource in the United States. Sandy beaches play an important role in maintaining water quality and safe access. Furthermore, a sandy beach makes a better ecotone (transitional environment) for flora and fauna than seawalls and revetments. Summary arguments supporting a sandy beach system include:

- 1) Beaches are filters for non-point source runoff.
- 2) Beaches reduce lakebed downcutting, a source of fine clay pollutants.
- 3) Beaches support endangered species such as sea rocket, marram grass, and seaside spurge.
- 4) Beaches make better wildlife habitat than actively eroding bluffs or seawalls.
- 5) Stone headlands make better fish habitat than eroding lakebed clay.
- 6) Beaches protect the lakebed from erosion that causes larger stormwaves to impact the shore.
- 7) Beaches are far safer for swimmers and boaters than a coast lined with seawalls or revetments, especially in an emergency.

On urban coasts, more than 35 years of system monitoring (Shabica et al, 2011) has shown that engineered pocket beaches (aka bay-beaches or attached-breakwater beaches), pre-nourished with sand, have shown a great resilience to changing lake-levels and decreased sediment-supply. After an intense storm such as the storm on Halloween, 2014, pocket beach recovery is fast. Further, net sand loss and renourishment costs are lower than for unprotected beaches on open Great Lakes coasts. And finally, a diverse coastal ecosystem dominated by American Beach Grass and native species like Sea-Rocket has been surprising resilient after severe shore erosion events. Planting of native species on the new beaches further improves the decreasing terrestrial habitat regionally. And with each beach, thousands of tons of new sand is brought in, not only to initially nourish the pocket beach but also to add 20% overfill sand to the adjacent lakeshore. Periodic sand re-nourishment has proven to be a successful management tool and provides additional sand for the entire Illinois coastal ecosystem.

**Impact to Littoral Drift System**

The proposed plan for this site includes construction of a breakwater-protected beach system and placement of mitigational sandfill, as required for permit. The design of the proposed system, including the mitigational sandfill, will help assure no negative impact to the littoral drift system. The existing section of Lake Michigan shoreline at 391 and 411 Sheridan Road, Winnetka is completely engineered. This section of the coastline consists of breakwater-held beaches, groins and revetments. Sand mitigation (as required by the IDNR) will be placed within the system with a 20% overfill as required.

The proposed quarystone breakwaters will extend less than 125' offshore, the southernmost breakwater will be shortened and maintained within its current footprint (extending to approximately 140' offshore). The proposed structures have been designed to fit in with the adjacent structures. The littoral drift system should remain at an equilibrium once the mitigational sand is placed (anticipated quantity plus 20% overfill).

IDNR regulations for structures that will retain sand require pre- and post-construction surveys, as well as surveys at the one and five-year intervals. This requirement will help assure that a sand equilibrium is met and that the new project is gaining and losing sand at a similar rate to neighboring properties or mitigation may be required at the owner's expense.

**Impact on Public Uses**

Public access will not be impacted implemented as the proposed work is all offshore with the exception of the revetment in the north cell. The beach will provide a safe place for boaters and swimmers in distress. Fishing will not be impacted negatively, as the underwater area of the quarystone protection will create an improved

fish habitat. Additionally, navigation of water craft will not be impacted, as the proposed construction will not extend further east than the existing structure immediately to the north.

### **Impact on Natural Resources**

Quarrystone structures in the nearshore waters of Lake Michigan and sandy beaches improve native species habitat. The LandOwner Resource Centre with support from the Canadian Wildlife Service and the Ontario Ministry of Natural Resources states that, “unstable shorelines can release silt that can choke nearby aquatic habitats.” Additionally, underwater structures such as artificial reefs constructed of large boulders and clean riprap material “in large water bodies, such as the Great Lakes . . . are often the best method of creating habitat.” As stated above, according to Meadows, et al., 2005, “a nearshore area with 100% sand cover support[s] a species rich community.” As the design does not impact the bluff and vegetation, the local terrestrial wildlife will continue to inhabit this property.

### **Type of Permit**

The scope of this project requires an individual permit.

### **Description and Schedule of Proposed Activity**

This project will be completed via marine construction with a barge and crane or backhoe delivering all materials and equipment to the site. The proposed work will be completed using a backhoe that will work from the beach to place the materials. Work will not begin until all necessary permits have been received. This work will require approximately 14 weeks to complete.

### **Type and Quantity of Fill/Measures Taken to Avoid Impact/Erosion and Sediment Control Plan**

All material will be clean and from inland quarries. Approximately 1,160 cubic yards of clean quarried stone will be placed to construct the breakwater. Approximately 2,962 cu. yds of clean sand will be placed as sandfill in and around the system. All clay removed from the lakebed for proper placement of the toe stone will be placed on a barge and disposed of properly.

The amount of fill to be placed below the Ordinary High Water Mark (581.5 feet, IGLD 1985) is +/- 0.13 acres.

### **Summary**

All of the above described activities and plans will follow IPP terms and conditions. All of the proposed work adheres to the guidelines prescribed by the Illinois Environmental Protection Agency and its Anti-Degradation Assessment. U.S. Fish & Wildlife Service and the Illinois Historic Preservation Association will be updated on all relevant correspondence.

If you have any questions please feel free to call me at the phone number below.

Sincerely,

Jon Shabica  
Vice President

cc: IDNR, Water Resources (Casey)  
IEPA (Heacock)

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www.shabica.com

**COVER LETTER**

391 & 411 Sheridan Road, Winnetka – January 19, 2016

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U.S. Fish & Wildlife Service  
Illinois Historic Preservation Agency (Haaker)  
Michael Hara

## DESIGN OF SHORELINE EROSION PROTECTION

### Introduction

The following report summarizes assumptions and design criteria for a quarystone breakwater system and sandfill to help retain a beach, provide lake access, and better protect the property located at 391 and 411 Sheridan Road, Winnetka. The design is based on the drawings included in the permit application to the U.S. Army Corps of Engineers dated October 30, 2015.

The site lies within a fully-engineered section of urban lakeshore that is typically protected with steel sheetpile groins, revetments and breakwater-protected beaches. This section of coast is sand-starved due to municipal structures (littoral barriers) constructed over the past 100 years that extend east past the littoral zone and reduce sand bypass, as well as armoring of the shoreline reducing erosion of the glacial clay bluffs. According to the Illinois State Geological Survey, there is almost no sand moving along this section of coast. All structures in the area have been steadily losing their effectiveness at holding beach sand. This problem is exacerbated by lakebed erosion. In many cases where all the sand has been lost, the adjacent bluffs have begun to erode. To provide adequate protection for the upland property, solutions have typically been of two types: breakwater- or groin-anchored beaches to protect the bluffs, or a lower-cost system with a lower level of protection in the form of quarystone revetments placed against the toe of the bluff that prevents stormwave erosion but at the expense of the beach and pedestrian access.

### Project Description

The proposed design includes designing a headland bay beach system with three shore-connected breakwaters (one of which is existing), the north breakwater extend southeast from the north neighbor's proposed breakwater, the central breakwater will form a "T" on the end of the existing steel sheetpile groin and the existing south breakwater will remain in place extending north from the south pier being shortened and maintained. This system will reduce the gap between the existing steel groins on the north section of the property, while the south beach area will be cleaned up removing extraneous rocks and concrete structures on the beach to leave a bay beach cell. The project will include sandfill mitigation that fulfills the design requirements of 20-year stormwave erosion protection. With the current elevated lake, stormwaves are causing severe erosion problems and impacts to the property, as well as may cause deeper water in the nearshore. The breakwater-protected beach system is designed to help retain and enhance the beach that occurs on this property.

### Summary Specifications

Using the Army Corps of Engineers Shore Protection Manual (1984), performance of nearby prototypes and other sources, the following specifications were developed for this site (elevations are based on IGLD 1985):

#### Breakwater Specifications

Lakeward Crest Elevation:	584 ft
Toe of Breakwater:	575 ft
Crest Width:	7 ft
Average Armor Size:	3.5 tons
"B" Stone	400 - 1200 lbs
Slope:	1:1.5
Tons/linear ft:	13.6 tons

**Assumptions**

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• Design High Water (DHW):	582.0 ft *
• Design Water Level:	580.0 ft
• Design Low Water (DLW):	577.5 ft *
• Existing clay till elevation at groin toe:	574.0 ft
• 20-yr lakebed erosion at toe of groin:	3 ft**
• Design wave height:	Hs = 10.8 ft
• Nearshore Slope:	1:20
• Design Wave Period (T):	9.9 s **
• Depth at Structure Toe DHW (Ds):	9.0'
• Design Deepwater Wave (Ho):	18.0'
• Design Wave Length (Lo):	501.8'
• Stone Porosity:	37%

\* DHW includes 2 ft storm setup, DLW is equivalent to Low Water Datum  
 \*\* Resio & Vincent, 1976

**Bathymetry**

Bathymetric surveying was performed in November 7, 2011. Six transects were completed in the project area and on adjacent properties. The survey data points were taken to approximately 300 feet east from the seawall. The survey was performed using an electronic total station with a diver in the water and a licensed survey crew on land. Reference benchmark: Per Monument Records for the Center of Section 21-42-13 on file with the Village of Winnetka, benchmark is the top of brass plug in concrete witness monument in the north parkway of Willow Road near the east line of Residence #467. Elevation = 613.21' NAVD 1988 Datum.

Topographic surveys of the lakefront and beach area were updated on August 4, 2014 and again on May 28, 2015 which show a severe deflation of the beach and erosion to the lakefront.

**Water Levels**

The following table summarizes water level data representing daily highest extremes measured at Calumet Harbor, Illinois, approximately 24 miles to the south of Winnetka. Note: Low water datum LWD = 577.5 ft (IGLD 1985).

Lake Level	LWD	IGLD 1985
Record High	+5.5	583.0
Record Low	-1.4	576.1

**Project Supporting Data**

To help facilitate project review, Shabica & Associates offers the following supporting data based on standard coastal engineering practices:

**1. Sediment transport around structure**

The structure is designed to lie within the surf zone (zone of breaking waves), therefore allowing sediment transport around the structure. The range of breaking wave heights is from 8.3 ft based on a 6-second wave with a wave length of 184 ft (using 1/25 Lo) to 18 ft based on a 9.9-second wave with a wave length of 501.8 ft (Resio and Vincent, 1976). The commonly accepted zone of sediment transport is to 18 ft (depth of closure) in this section of Lake Michigan, which is a function of the design wave parameters. Based on

this data, once the structure has been filled with sand, it will continue to bypass littoral drift sand. Rod and transit survey monitoring will be conducted, as required by the IDNR, to assure that the system performs as designed.

The IDNR requires sandfill in areas where sediment will be trapped by the new system. Sand volume quantities have been calculated as shown in the permit drawings. As required by the IDNR, a 20% overflow will be added to the calculated volume. Additionally, the new pre- and post-construction monitoring will be performed and submitted to the IDNR to verify the impacts to the system.

## 2. Effect on Adjacent Shorelines

A wave diffraction diagram (Figure 2, Appendix) has been overlain on the proposed shore protection system. Using a refracted incident wave angle of 90 degrees (USACE, Shore Protection Manual), with average and design waves, there will be a decrease in wave energy on adjacent properties. The wave diffraction pattern shows that the coefficient of diffraction (K) reduces the wave energy to a distance of about  $\frac{1}{2}$  the wave length downdrift and does not have an impact further downdrift. For the average 6-second wave, that distance of reduced wave energy is about 90 ft and for the design wave, the protected distance is about 250 ft. This protected area close to the structure has diminished wave energy that will in turn reduce erosion in the area.

## 3. Wave Reduction in Rubble-Mound Structures

The Iribarren number ( $\xi$ ), or surf similarity number, is used to determine the wave reflection coefficient. For rubble-mound structures, wave reflection (and wave energy) is reduced by one half or more (0.2 to 0.53) (Figure 3, Appendix). For example, a wave reflection of 0.25 means that the wave energy is reduced by 75%. The range of wave reflection for beaches peaks at about 0.44. The range for plane slopes, however, quickly rises to 0.5 and peaks at .91. This illustrates that rubble-mound structures reduce wave energy almost as well as beaches.

### **Lakebed Erosion**

Lakebed erosion, active in water depths of 10 ft or less, is a design component of this plan. This section of Winnetka lakeshore is considered sediment-starved. Sand deposits were measured near this site (Elder Lane in Winnetka). Sand deposits were thin to a distance of 160 ft from shore (Shabica, 2010) and sand was non-existent in areas to a distance of over 1000 ft from shore (Shabica & Pranschke, 1994). Also, the site is underlain by highly-erodible, cohesive glacial clay-till. This condition increases the rate of irreversible lakebed erosion that causes deepening of the water and larger waves to impact the shoreline. According to Robert Nairn, approximately 200 m<sup>3</sup> of sand cover per meter of lakeshore (out to a depth of 4 m) is necessary to protect the underlying cohesive profile from lakebed erosion under most conditions. Sand and coarser sediments represent typically less than 15% of the material eroding from the lakebed and bluffs.

Using the historic rate of lakebed downcutting of 0.15 ft/yr, an irreversible lowering of the nearshore lakebed clay of approximately 3.0 ft over a 20-year period is predicted in unprotected areas. With the breakwater and sandfill installed, the lakebed erosion will be reduced.

### **Stone Stability, Armorstone**

The proposed shore parallel section of the quarystone breakwater will have two layers of 2 to 5-ton armorstone placed on a 1:1.5 slope. Overtopping of the breakwater is expected during high water levels. Design conditions include:

- Lakeward breakwater crest elevation at 2.0 ft above DHW, 6.5 ft above DLW
- Depth-limited breaking waves will break on the stone groin and sand beach

- Depth at the toe of the structure is 8 ft (574.0) at DHW
- Incident wave directions: NE, E and SE
- Wave period for DHW:  $T = 9.9$  seconds
- Wave period for average conditions  $T = 6$  seconds

Quartzite armorstone is recommended as it is highly durable and is locally available in most gradations under 6 tons. Hudson's formula was used to estimate armorstone size. Using the design conditions, an armorstone of 3.8 tons is predicted for single layer special placement.

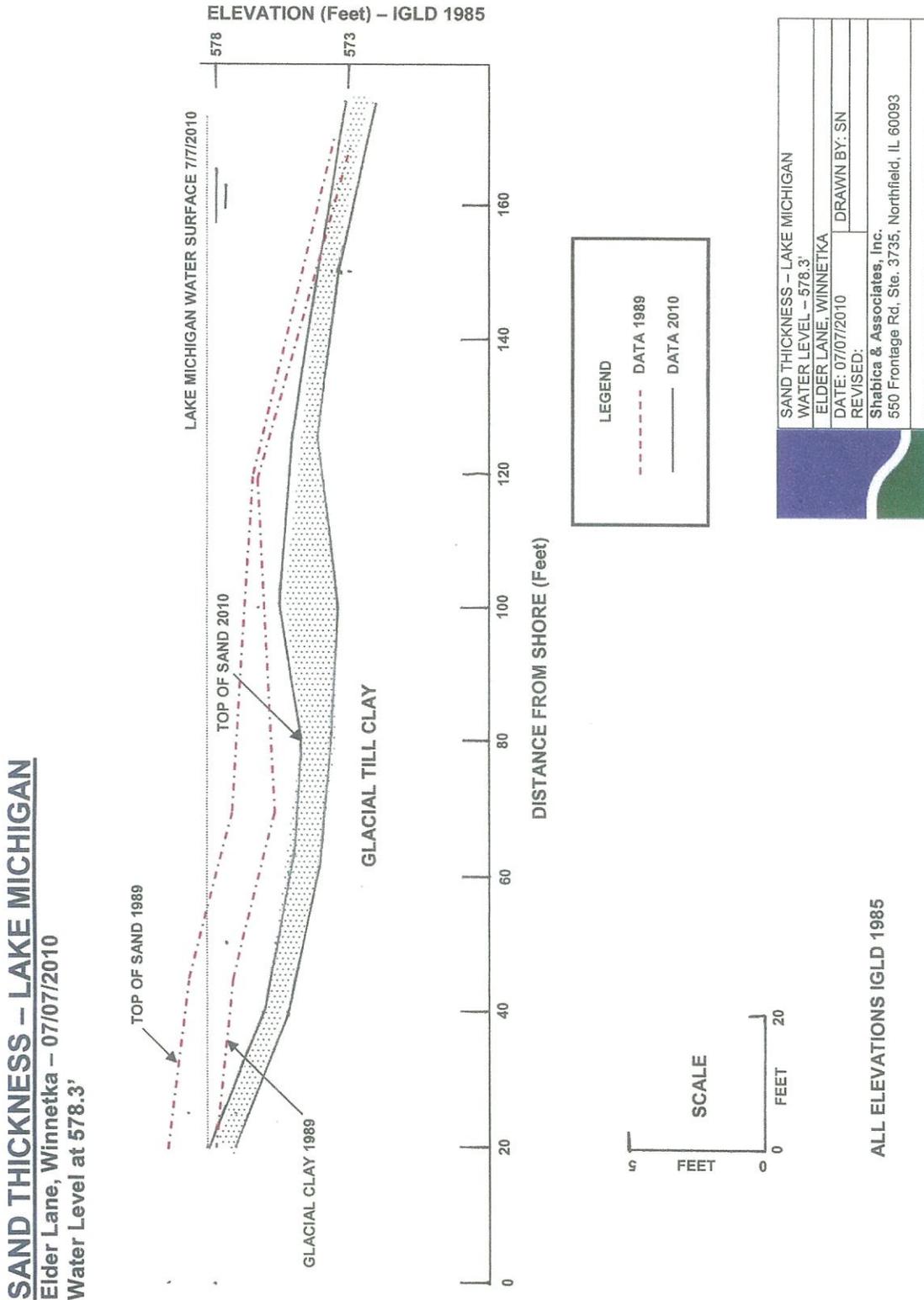
**Project Monitoring**

As the performance of shore protection structures cannot be predicted with absolute certainty, the shore protection system for 391 and 411 Sheridan Road, Winnetka, will be inspected as required by IDNR guidelines. This includes topographic and hydrographic surveys beginning at an elevation of 581.5 feet (IGLD 1985) and progressing to 300 feet lakeward of the lakeward end of the project within the north and south property lines. Additionally, all structures should be inspected to assure that they continue to meet design specifications.

## References

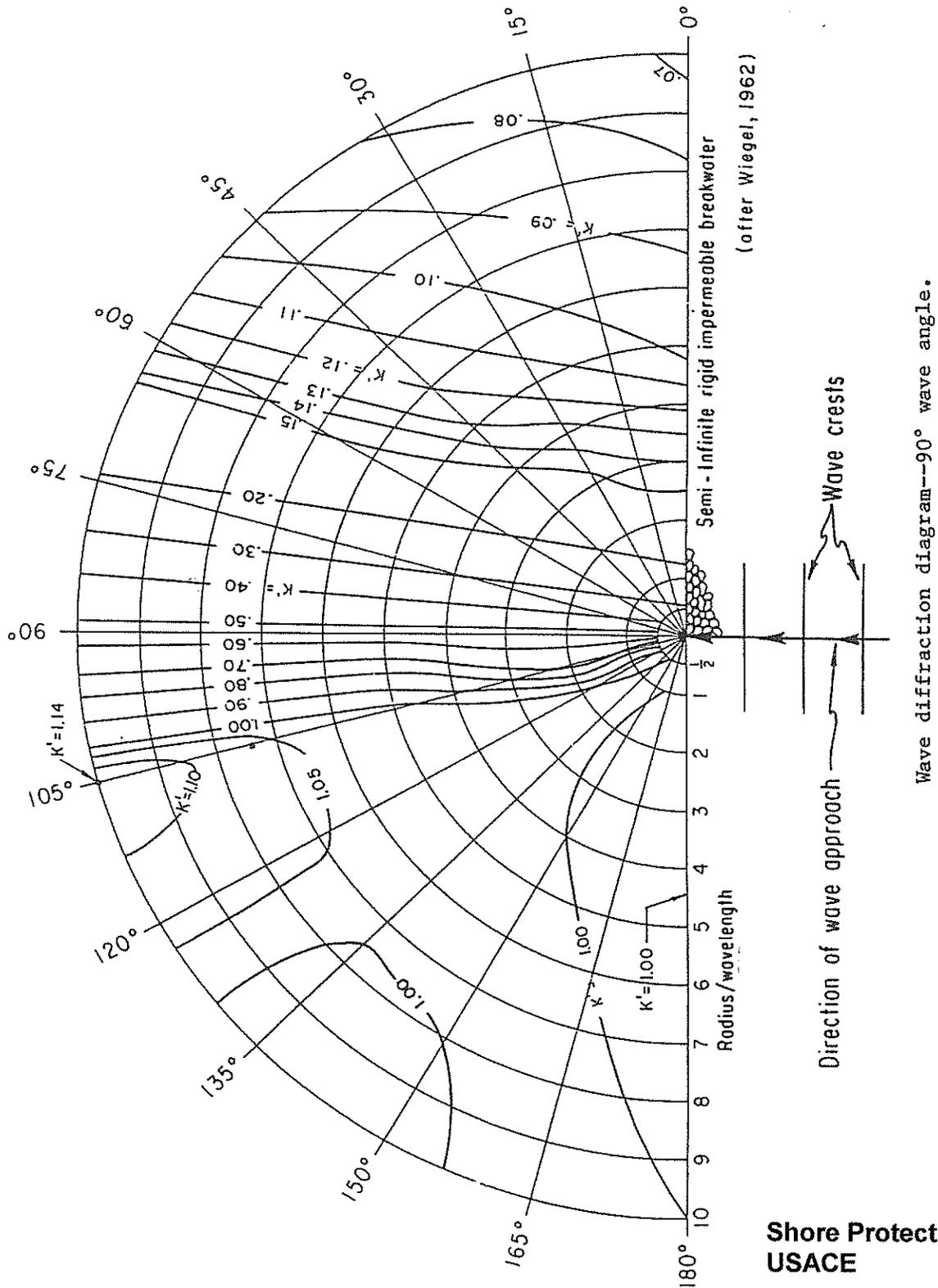
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**FIGURE 1**



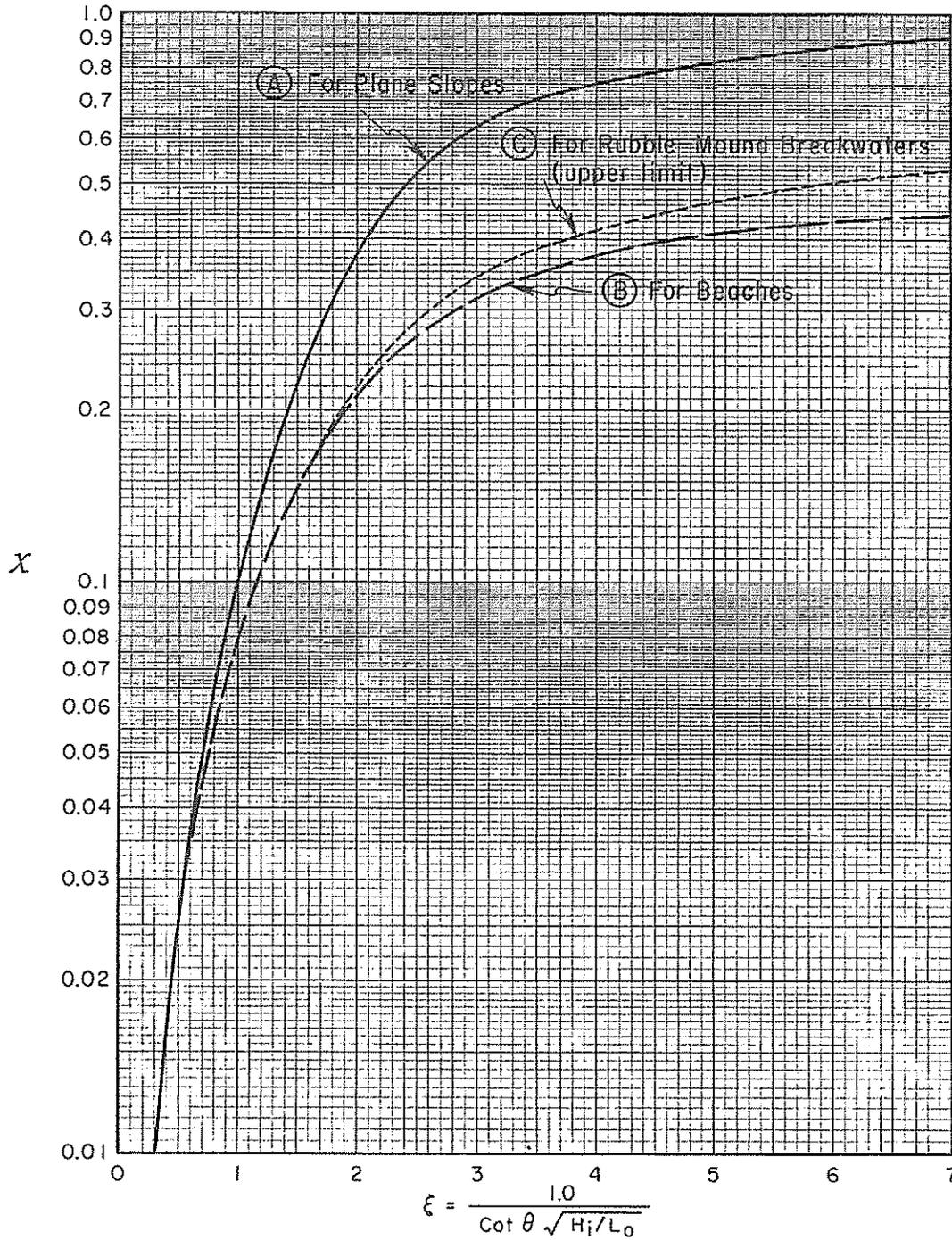
Typical cross-section of Winnetka lakebed and sand cover  
 (From Shabica et al., 1991)

FIGURE 2



Shore Protection Manual  
USACE

**FIGURE 3**



Wave reflection coefficients for slopes, beaches, and rubble-mound breakwaters as a function of the surf similarity parameter  $\xi$ .

**Shore Protection Manual  
 USACE**



2015 Google Earth Photo;  
see approximate  
property lines



2015 North beach cell with  
narrow beach during lake  
levels 1 ft over LWD. During  
even small lake storms, the  
concrete structures are  
vulnerable to wave attack  
and undermining.



2015 South beach cell with  
cobble beach. Concrete  
modular structures inhibit  
pedestrian access to the  
lake. Stone pile in the center  
of the beach (right side of  
photograph) hold a perched  
beach landward.



**8. PROJECT DESCRIPTION (Include all features):**

The shoreline stabilization will be comprised of installing quarystone breakwaters that are connected to existing structures (or proposed structures already under review) to create a cohesive bay beach system with revetments prudently located. No new work will be done further than 125' east of the bluff toe. The project includes removal of several previously designed structures that are oddly located and in some cases impede access to Lake Michigan. Beginning at the north, a breakwater spur will be constructed extending southeast from the currently proposed breakwater on the north property (419 Sheridan). This breakwater will have a crest of 583' and slopes of 1:1.5. Sections of quarystone revetment will be constructed lakeward of the easternmost retaining walls as necessary for shore protection. Revetment crests will be up to the elevation of the retaining walls not to exceed 588' with a slope of 1:1. A quarystone breakwater will be constructed extending north and south from the existing steel groin between the 391 and 411 lots. This breakwater will be 114' long (52' north and 62' south from the steel groin to the toe stone). The breakwater crest will be 584' with slopes of 1:1.5. Three structures on the 391 lot will be removed: the north breakwater, the concrete modular structures and the piles of armorstone in the middle of the beach that hold a perched beach. The south breakwater that extends north from the existing pier will be shortened and maintained. This structure does extend beyond 125' east from the shoreline, but its length will be reduced and the structure maintained to provide long-term stability. The crest will be reduced from a high of 588.5' to 586' tapering to 584' with slopes of 1:1.5. The revetment on the 391 lot will be maintained as necessary simply to fill void spaces and provide long-term stability. Approximately 2962 cu. yds of clean sand will be placed as required by the IDNR. Additionally, the applicant is requesting approval for a 10-year sand nourishment permit to bring in up to 1000 tons of sand annually, as needed.

**9. PURPOSE AND NEED OF PROJECT:**

To maintain a stable beach and lakebed as well as protect the existing structures built into the bluff.

**COMPLETE THE FOLLOWING FOUR BLOCKS IF DREDGED AND/OR FILL MATERIAL IS TO BE DISCHARGED**

**10. REASON(S) FOR DISCHARGE:**

To provide adequate shore protection within an engineered section of coastline.

**11. TYPE(S) OF MATERIAL BEING DISCHARGED AND THE AMOUNT OF EACH TYPE IN CUBIC YARDS FOR WATERWAYS:**

TYPE: Stone and sand  
 AMOUNT IN CUBIC YARDS:  
 Stone: 1160 cu. yds; Sand: 2962 cu. yds.

**12. SURFACE AREA IN ACRES OF WETLANDS OR OTHER WATERS FILLED (See Instructions)**

New stone will cover +/- 0.13 acres

**13. DESCRIPTION OF AVOIDANCE, MINIMIZATION AND COMPENSATION (See instructions)**

Existing infrastructure will be utilized in the proposed plan. Extraneous structures on the 391 lot will be removed. Wetland Mitigation Banking Credits will be purchased according to regulatory requirement.

**14. Date activity is proposed to commence**

August 1, 2016

**Date activity is expected to be completed**

14 weeks

15. Is any portion of the activity for which authorization is sought now complete?  Yes  No  
 Month and Year the activity was completed

Yes  No

NOTE: If answer is "YES" give reasons in the Project Description and Remarks section. Indicate the existing work on drawings.

16. List all approvals or certification and denials received from other Federal, interstate, state, or local agencies for structures, construction, discharges or other activities described in this application.

<u>Issuing Agency</u>	<u>Type of Approval</u>	<u>Identification No.</u>	<u>Date of Application</u>	<u>Date of Approval</u>	<u>Date of Denial</u>

**17. CONSENT TO ENTER PROPERTY LISTED IN PART 7 ABOVE IS HEREBY GRANTED.**

Yes  No

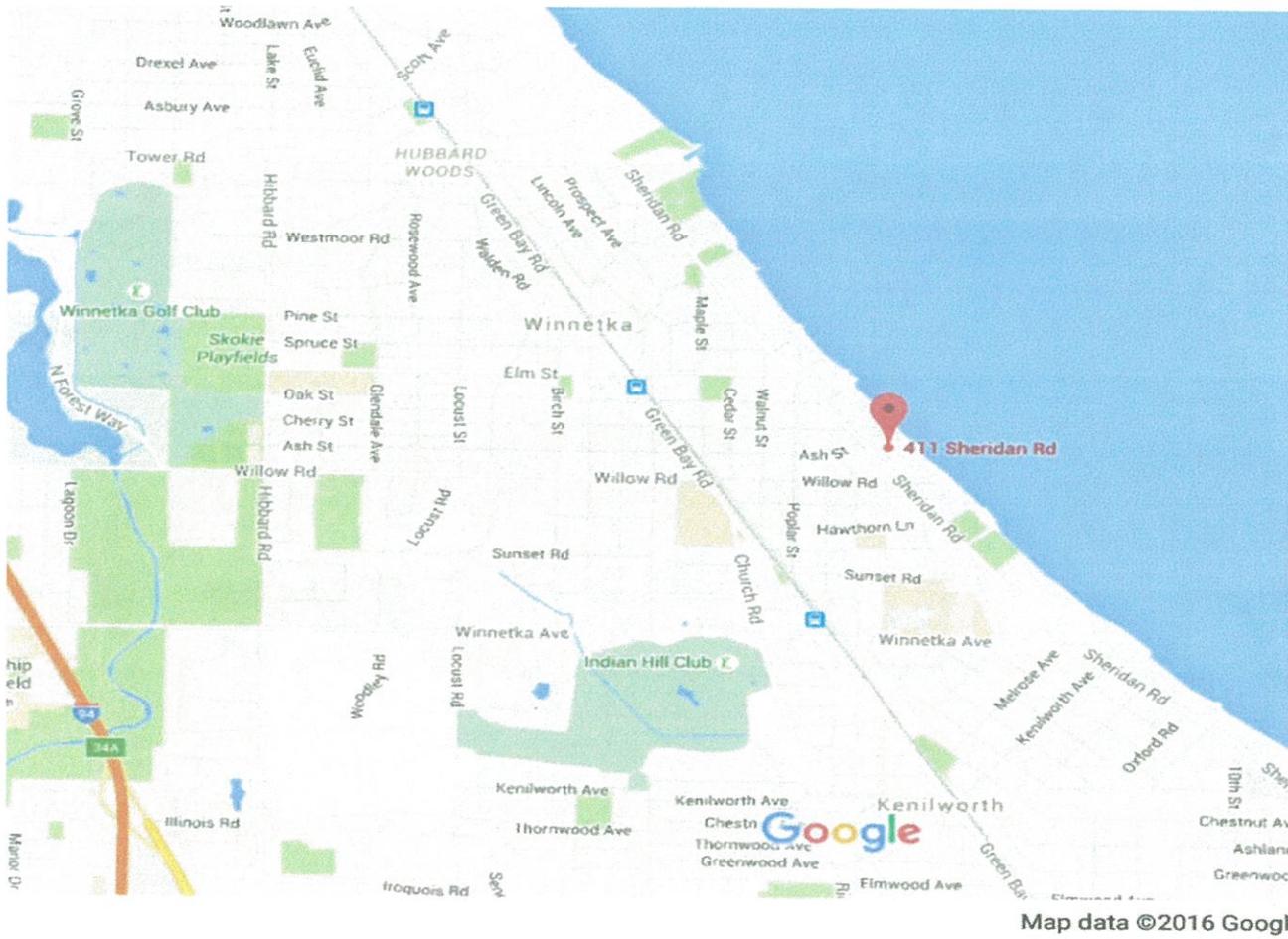
**18. APPLICATION VERIFICATION (SEE SPECIAL INSTRUCTIONS)**

Application is hereby made for the activities described herein. I certify that I am familiar with the information contained in the application, and that to the best of my knowledge and belief, such information is true, complete, and accurate. I further certify that I possess the authority to undertake the proposed activities.

_____ Signature of Applicant or Authorized Agent	_____ Date
_____ Signature of Applicant or Authorized Agent	_____ Date
_____ Signature of Applicant or Authorized Agent	_____ Date

- Corps of Engineers Revised 2010     IL Dep't of Natural Resources     IL Environmental Protection Agency     Applicant's Copy

# Vicinity Map



Breakwater-Protected Beach

391 and 411 Sheridan Road  
Winnetka, IL 60093



**Shabica & Associates, Inc.**  
WE BUILD BEACHES

**Title:** Construction of a quarystone breakwater along Lake Michigan  
Michael Hara  
391 and 411 Sheridan Road  
Winnetka, Illinois 60093

**Submittal Date:** January 19, 2016

**Plan Sheets:** 2015.10.22 Hara Existing Bathymetry – Sheet 1 of 6  
2015.10.22 Hara Plan View – Existing Structures – Sheet 2 of 6  
2015.10.19 Hara Plan View – Proposed Work – Sheet 3 of 6  
2015.10.22 Hara Cross Sections: Breakwater and Revetment – Sheet 4 of 6  
2015.10.19 Hara Plan View – Sand – Sheet 5 of 6  
2015.10.28 Hara Sand Calculations – Sheet 6 of 6

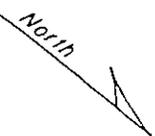


**Shabica & Associates, Inc.**  
WE BUILD BEACHES

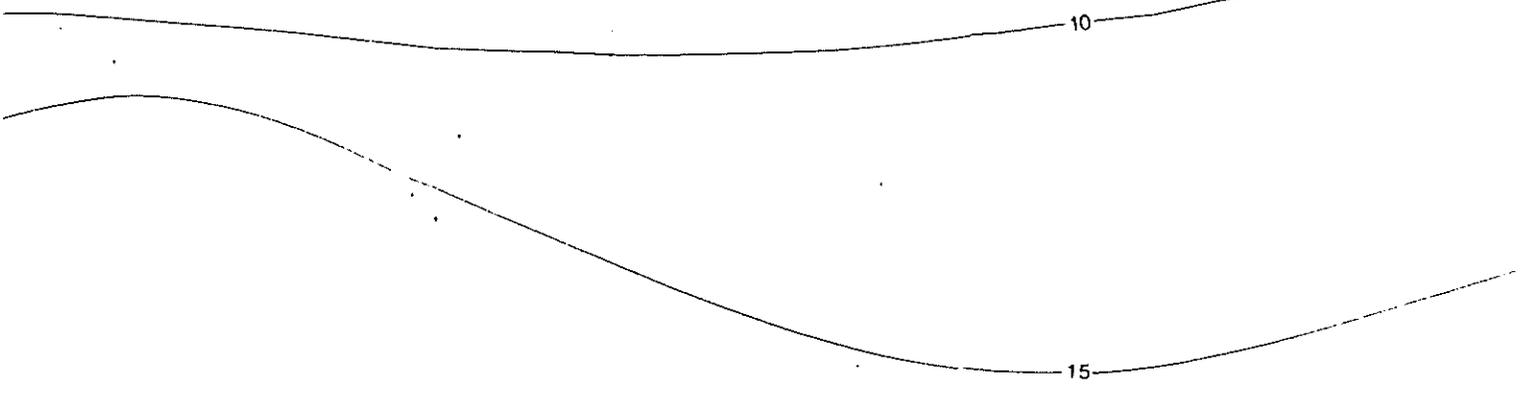
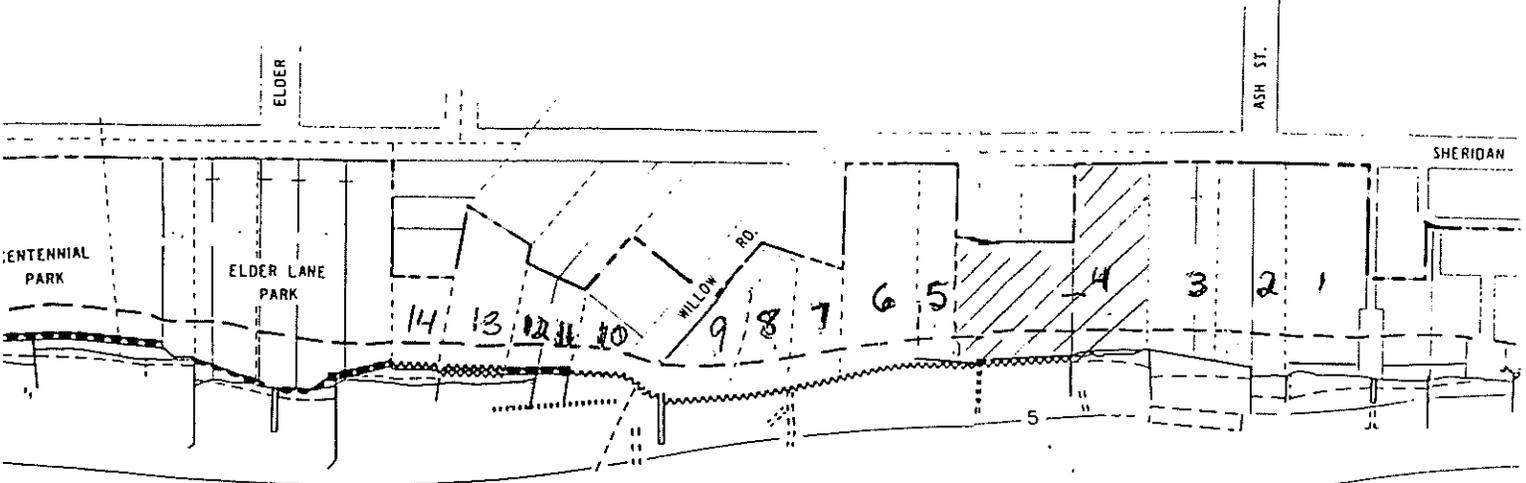
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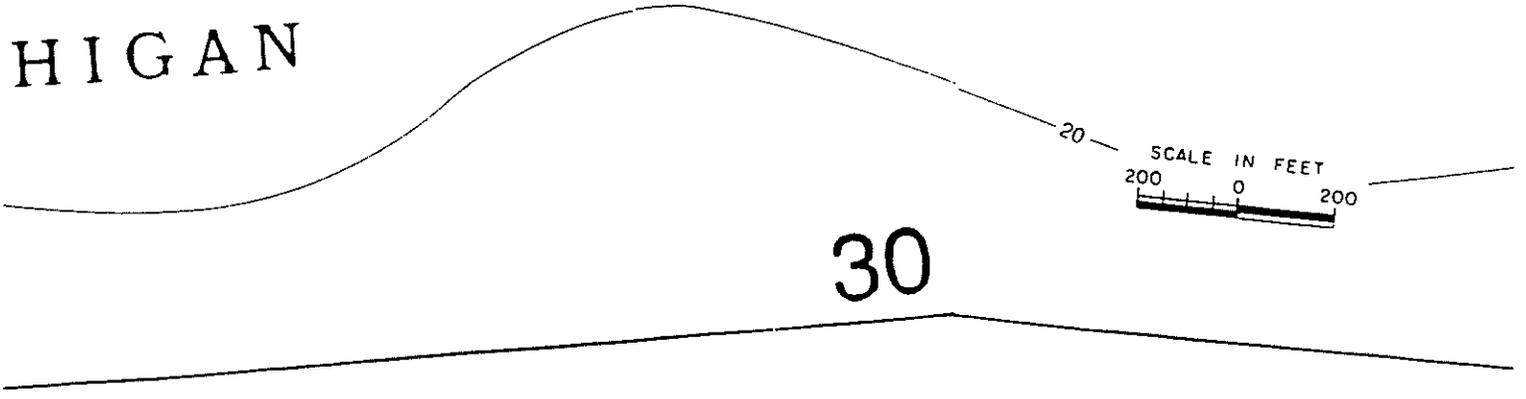
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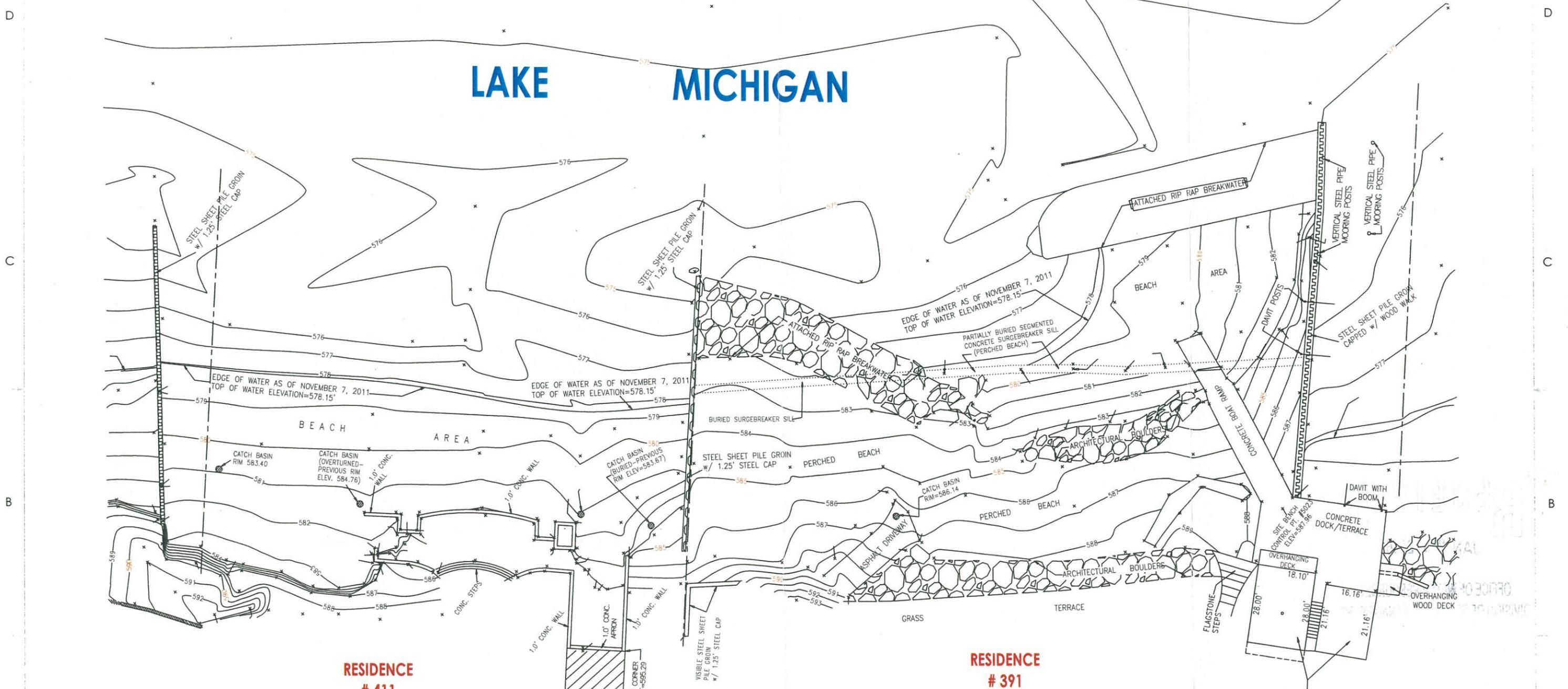
MINNETKA



HIGAN



**EXISTING BATHYMETRY**



**RESIDENCE # 411**

**RESIDENCE # 391**

NAME	DATE	Project Location:
SN	10/22/15	391 & 411 SHERIDAN, WINNETKA
JS	10/30/15	

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 Northfield, Illinois 60093  
 847-446-1436  
 www.shabica.com

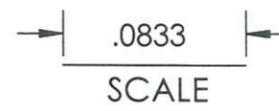
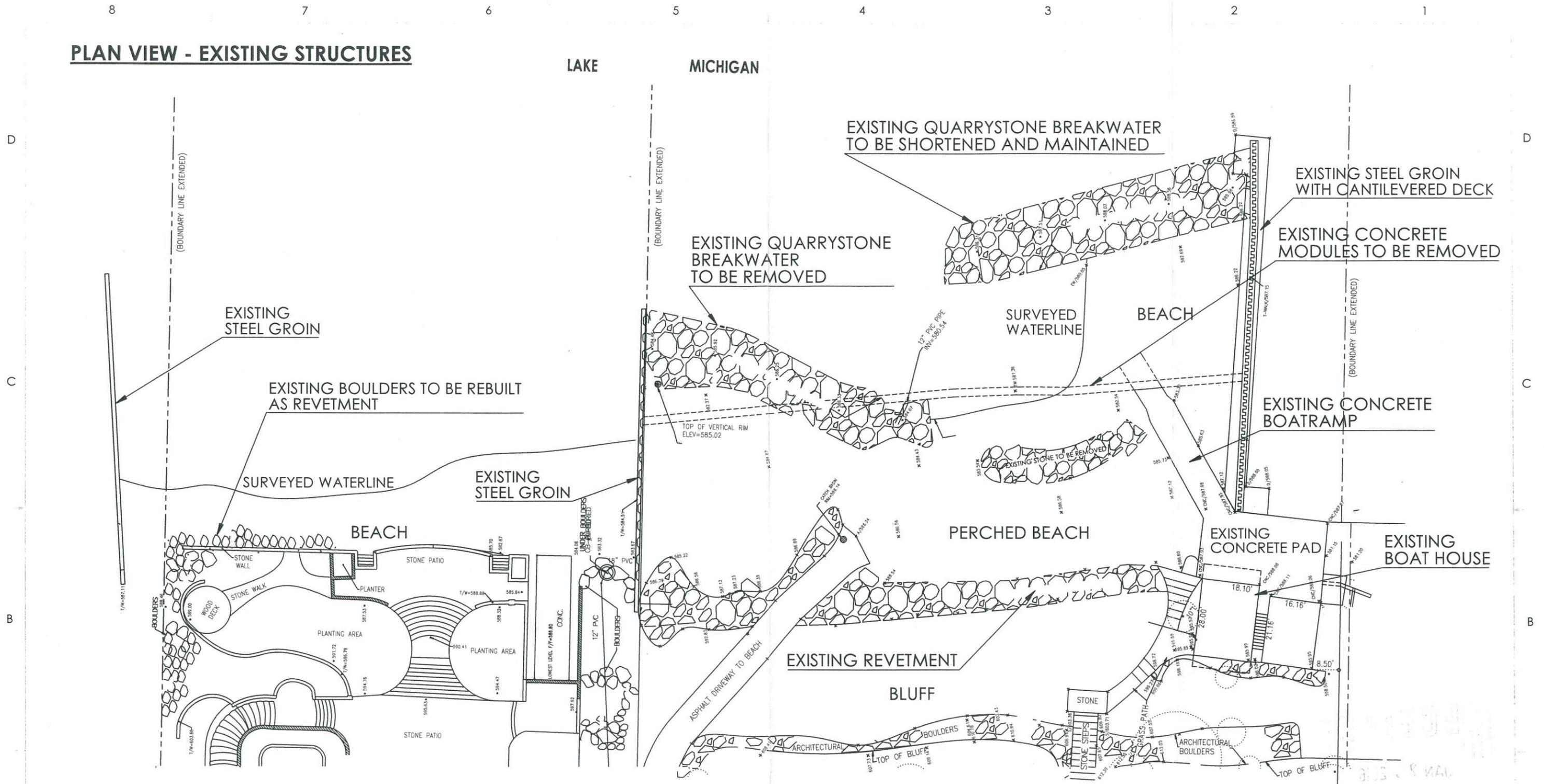
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 ALL ELEVATIONS IN IGLD 1985

SIZE **B** EXISTING BATHYMETRY REV

**PLAN VIEW - EXISTING STRUCTURES**



Project Location:  
 391 & 411 SHERIDAN, WINNETKA

NAME DATE  
 DRAWN SN 10/22/15  
 CHECKED JS 10/30/15

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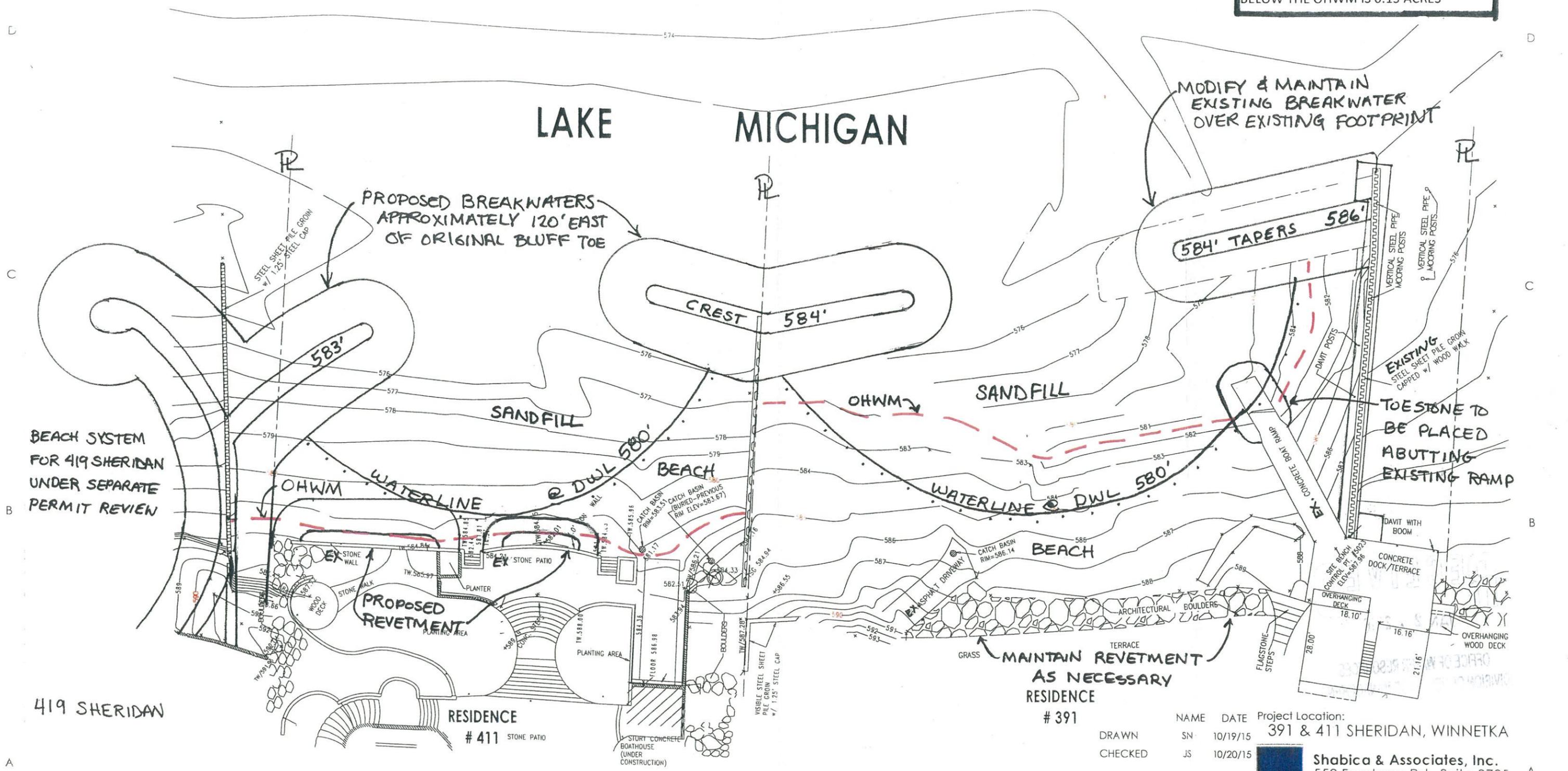
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 REV

PLAN VIEW

COVERAGE OF NEW FILL ON THE LAKEBED BELOW THE OHWM IS 0.13 ACRES



BEACH SYSTEM FOR 419 SHERIDAN UNDER SEPARATE PERMIT REVIEW

419 SHERIDAN

RESIDENCE # 411 STONE PATIO

RESIDENCE # 391

DRAWN SN 10/19/15  
 CHECKED JS 10/20/15

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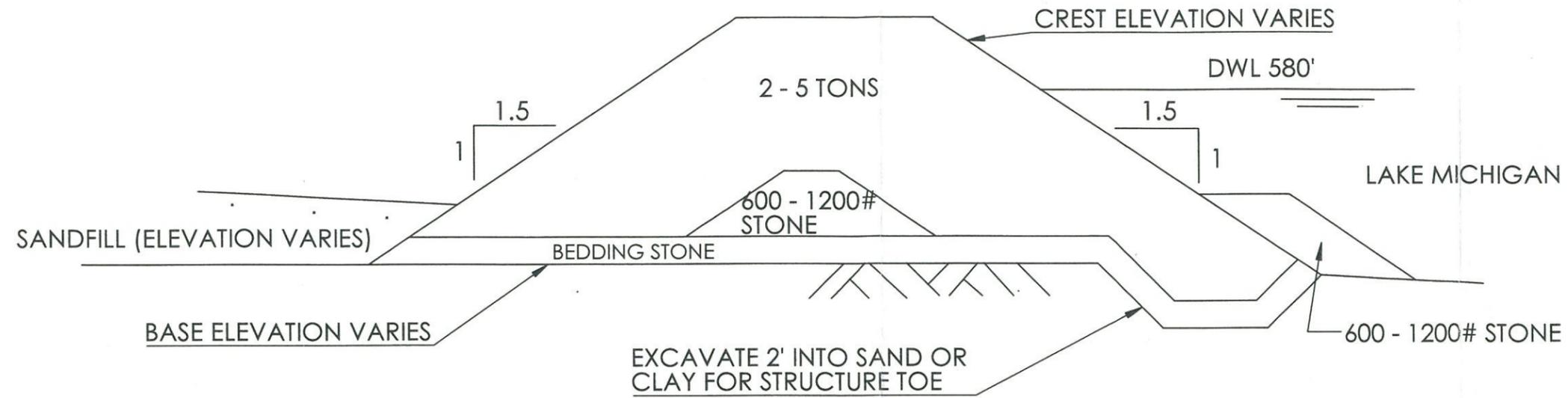
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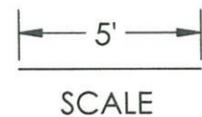
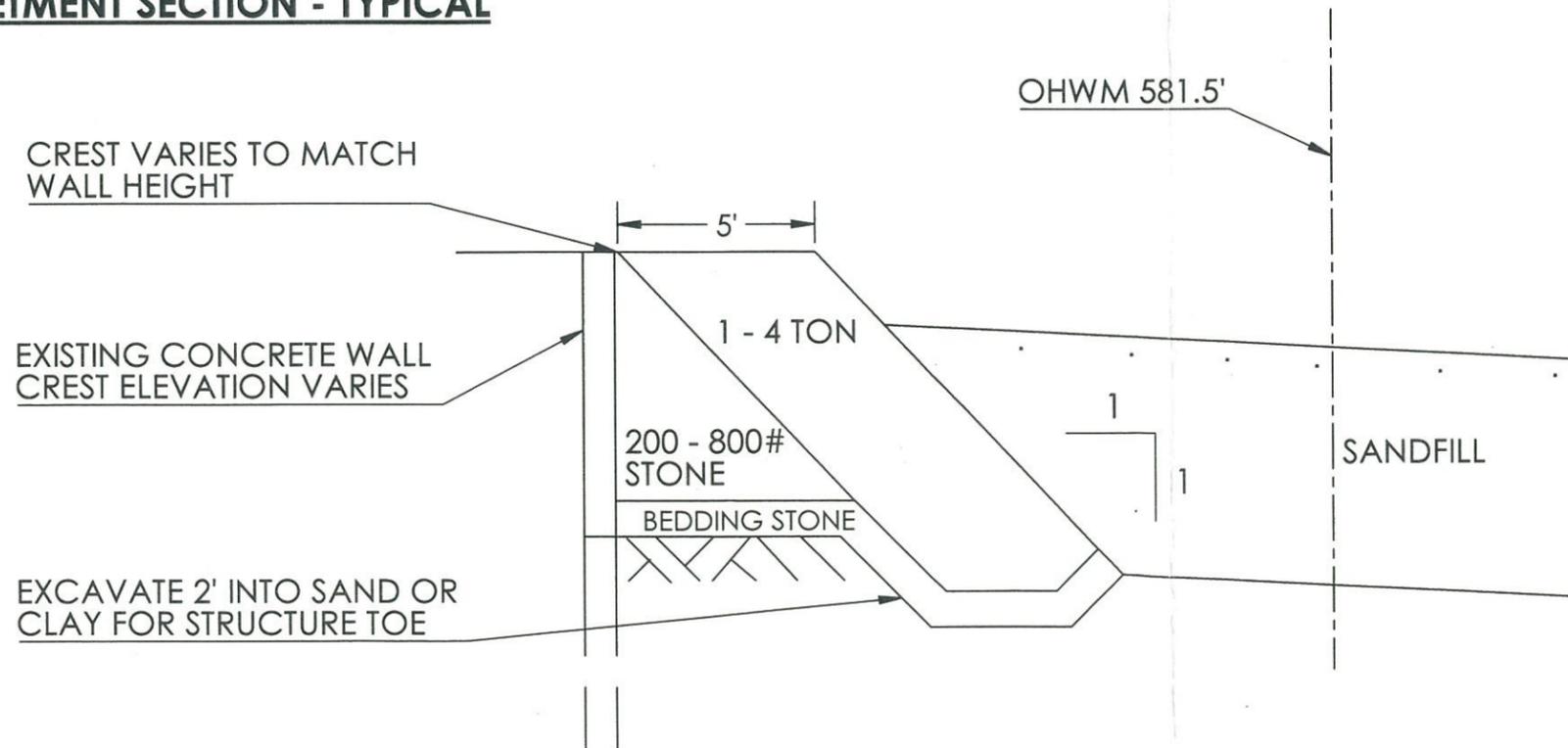
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SIZE **B**  
 PLAN VIEW PROPOSED WORK  
 REV

**BREAKWATER SECTION - TYPICAL**



**REVTMENT SECTION - TYPICAL**



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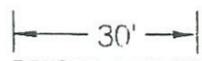
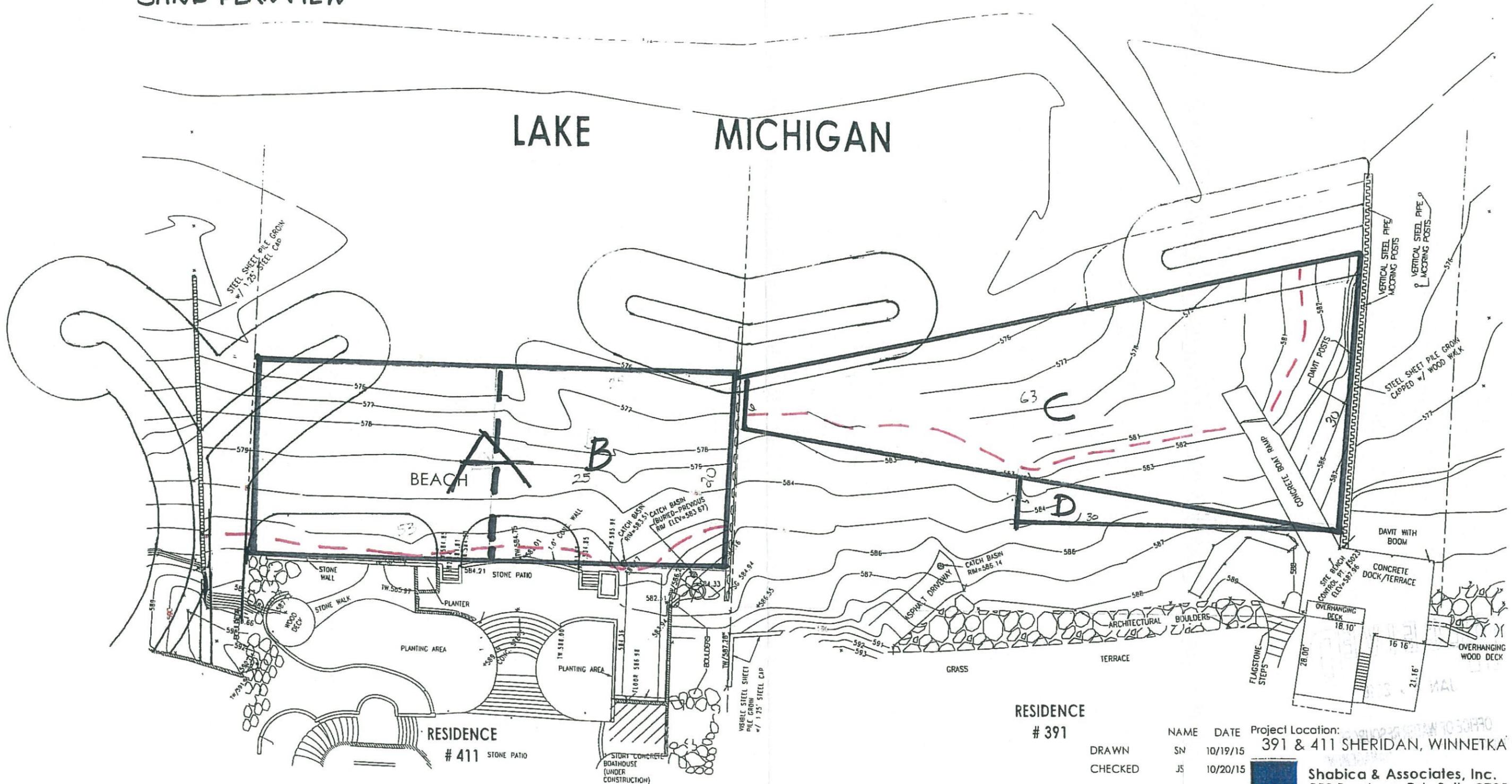
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 ALL ELEVATIONS IN IGLD 1985

SIZE  
**B**  
 CROSS SECTIONS:  
 BREAKWATER  
 AND REVTMENT  
 REV

SAND PLANVIEW

LAKE MICHIGAN



RESIDENCE # 391

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NAME DATE Project Location:  
 SN 10/19/15 391 & 411 SHERIDAN, WINNETKA  
 JS 10/20/15

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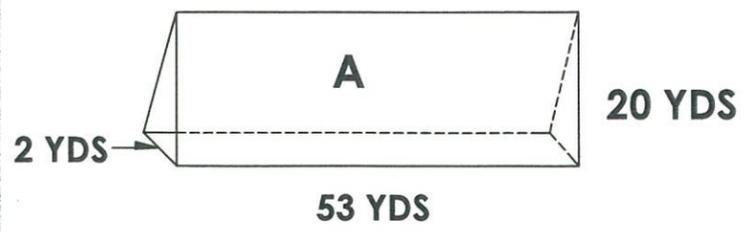
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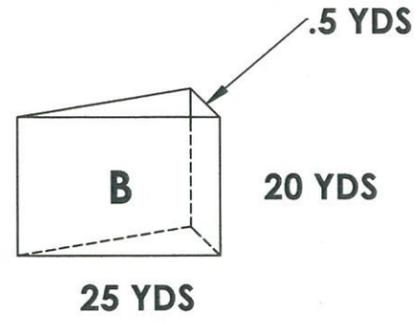
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SIZE B

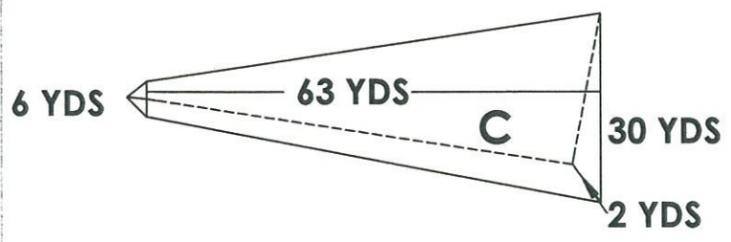
PLAN VIEW - SAND REV



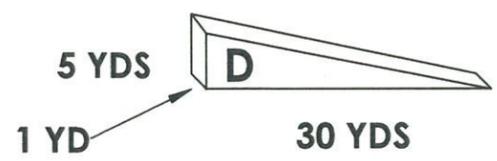
$$\frac{20 \text{ YDS} \times 53 \text{ YDS} \times 2 \text{ YDS}}{2} = 1060 \text{ YDS}$$



$$\frac{20 \text{ YDS} \times 25 \text{ YDS} \times 0.5 \text{ YDS}}{2} = 125 \text{ YDS}$$



$$\frac{18 \text{ YDS}^* \times 63 \text{ YDS} \times 2 \text{ YDS}}{2} = 1134 \text{ YDS}$$



$$5 \text{ YDS} \times 30 \text{ YDS} \times 1 \text{ YD} = 150 \text{ YDS}$$

1060 + 125 + 1134 + 150 = 2469 CU YDS  
 2469 CU YDS X 1.25 CU YDS/TON = 3,086 TONS  
 3,086 TONS X 20% OVERFILL = 617 TONS  
 3,086 TONS + 617 TONS = 3703 TONS

**PLACE 3,720 TONS OF CLEAN SANDFILL**

\* AVERAGE 6 AND 30 TO COME UP WITH 18 YDS

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NAME SN JS  
DATE 10/28/15 11/2/15

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Project Location:  
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SIZE **B** SAND CALCULATIONS REV